# Multilayer Analytic Element Modeling of Radial Collector Wells

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(1) Yep, same guy

#### Outline

How Multilayer Systems Ought To Work
How Radial Collector Wells In A Multilayer Aquifer Ought To Work
Hooray, a Use For that 3D Model!
Proof that Bigger Isn't Always Better

#### What?

- Separate aquifer into individually homogeneous horizontal layers
  One layer contains RCW
  Arms of RCW modeled with multiaquifer line sinks
- RCW complexities incorporated

# Why?

Horizontal flow modeled analytically
Vertical stratification / anisotropy
BCs such as friction, head losses
Can be combined with regional 2D model

# Analytic Multilayer Approach

Divide aquifer vertically into *N* individually homogeneous layers (D-F approx in each)
Transmission between layers governed by:

vertical discharge from center to center
vertical hydraulic conductivity
vertical resistance

Net result: system of *N* differential equations.....

# Multilayer System Equations

$$q_{z,i} = \frac{h_i - h_{i-1}}{c_i}$$

$$c_i = \frac{H_{i-1}}{2k_{v,i-1}} + \frac{H_i}{2k_{v,i}}$$

$$\nabla^2 h_i = \frac{q_{z,i} - q_{z,i+1}}{T_i}$$

$$\nabla^2 h_i = \frac{h_i - h_{i-1}}{c_i T_i} - \frac{h_{i+1} - h_i}{c_{i+1} T_i}$$

#### The Radial Collector Well

- The RCW is in one of the N layers
- Thickness of this layer = diameter of RCW arms
- Each arm modeled by M multiaquifer line sinks (constant strength)
- Each sink of length L
- Discharge of arm j:

$$Q_j = \sum_{m=1}^M \sigma_{m,j} L_{m,j}$$

#### A Call To Arms

\$\overline{1}\$ head inside arm [L]
\$\overline{1}\$ head outside arm [L]
\$\overline{1}\$ c: resistance to inflow [T]

 $\sigma = q (2 \pi R) ; \qquad q = (h - \phi) / c$  $\Rightarrow \sigma = (h - \phi) / d ; \qquad d = c / (2 \pi R)$ 

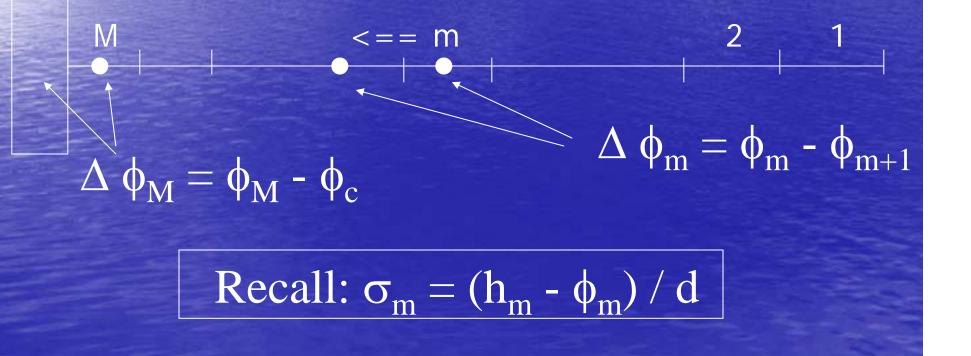
(h)

 $(\phi)$ 

#### Head Loss

 Quantified by comparison of conditions at the centers of adjacent line sinks

•  $\phi_m$  : head at center of line sink *m* 



For adjacent line sinks (m < M):  $\sigma_{\rm m} = (h_{\rm m} - \phi_{\rm m}) / d$  $\sigma_{m+1} = (h_{m+1} - \phi_{m+1}) / d$ Combine:  $d(\sigma_m - \sigma_{m+1}) = (h_m - h_{m+1}) - \Delta \phi_m$ Or  $(h_m - h_{m+1}) - d(\sigma_m - \sigma_{m+1}) = \Delta \phi_m$ 

# For link sink M $\sigma_{\rm M} = (h_{\rm M} - \phi_{\rm M}) / d$ ; $\Delta \phi_{\rm M} = \phi_{\rm M} - \phi_{\rm c}$ => $d\sigma_{\rm M} = h_{\rm M} - (\Delta \phi_{\rm M} + \phi_{\rm c})$ $h_{M} - d(\sigma_{M}) - \phi_{c} = \Delta \phi_{M}$

# Finding $\Delta \phi_m$ (for m < M)

• Darcy<sup>(2)</sup>-Weisbach for m < M

$$\Delta \phi_m = f \, \frac{l_m}{D} \frac{V_m^2}{2g}$$

$$\Rightarrow Q_{\rm m} = \pi R^2 V_{\rm m}$$

$$\Rightarrow V_{\rm m}^2 = Q_{\rm m}^2 / \pi^2 R^4$$

$$\Delta\phi_m = f \, \frac{l_m}{2\pi^2 R^5} \frac{Q_m^2}{2g}$$

(2) oh, nevermind

# Finding $\Delta \phi_m$ (for m = M)

• For m = M: (segment M)

(into caisson)

$$\Delta \phi_M = f \frac{l_M}{D} \frac{V_M^2}{2g}$$

$$\Delta\phi_C = K_L \frac{V_M^2}{2g}$$

 $V_{\rm m}^{2} = Q_{\rm m}^{2} / \pi^2 R^4$  ;  $K_{\rm L} = 1.0$ 

$$\Delta\phi_M = \left(f\frac{l_M}{2R} + 1\right)\frac{Q_M^2}{2g\pi^2 R^4}$$

#### Good Grief, Now We Need Q<sub>m</sub>

#### • For m < M ...

•  $S_m$ : total discharge at center of sink m  $S_m = \sigma_1 L_1 + ... + \sigma_{m-1}L_{m-1} + (\sigma_m L_m)/2$   $S_{m+1} = \sigma_1 L_1 + ... + \sigma_m L_m + (\sigma_{m+1} L_{m+1})/2$ •  $Q_m$ : average discharge,  $(S_m + S_{m+1}) / 2$  $Q_m = \sigma_1 L_1 + ... + \sigma_{m-1}L_{m-1} + (3\sigma_m L_m + \sigma_{m+1}L_{m+1})/4$   $Q_{\rm m}$  for  ${\rm m} = {\rm M}$ Compare center of sink M to end of arm at caisson:  $S_{M} = \sigma_{1} L_{1} + ... + \sigma_{M-1} L_{M-1} + (\sigma_{M} L_{M})/2$  $S_c = \sigma_1 L_1 + \dots + \sigma_M L_M$ •  $Q_M$ : average discharge,  $(S_M + S_c) / 2$  $Q_{\rm M} = \sigma_1 L_1 + \ldots + \sigma_{\rm m-1} L_{\rm m-1} +$  $3\sigma_{\rm M}L_{\rm M}/4$ 

Recap: For m < M,  $(h_m - h_{m+1}) - d(\sigma_m - \sigma_{m+1}) = \Delta \phi_m$  (\*\*)

where

$$\Delta\phi_m = f \frac{l_m}{2\pi^2 R^5} \frac{Q_m^2}{2g}$$

and

 $Q_{m} = \sigma_{1}L_{1} + \dots + \sigma_{m-1}L_{m-1} + (3\sigma_{m}L_{m} + \sigma_{m+1}L_{m+1})/4$ 

# Recap: For m = M, $h_M - d(\sigma_M) - \phi_c = \Delta \phi_M$ (\*\*)

where

$$\Delta\phi_M = \left(f\frac{l_M}{2R} + 1\right)\frac{Q_M^2}{2g\pi^2 R^4}$$

and

 $Q_{M} = \sigma_{1}L_{1} + \dots + \sigma_{m-1}L_{m-1} +$  $3\sigma_{\rm M}L_{\rm M}/4$ 

# Pop Quiz!

- Is the number of equations equal to the number of unknowns?
- No! The number of equations is balanced by the unknown line sink strengths σ, but the head in the caisson, φ<sub>c</sub>, is also unknown
- Need one extra equation. Let the total discharge of a well with P arms be Q<sub>w</sub>,

$$Q_w = \sum_{j=1}^{P} Q_j = \sum_{j=1}^{P} \sum_{m=1}^{M} \sigma_{m,j} L_{m,j}$$

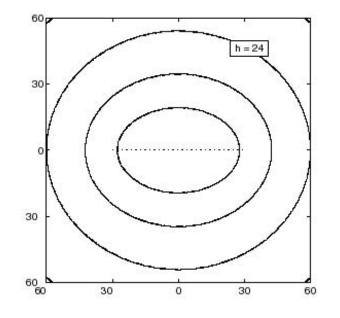
# Pop Quiz, continued

- Are our equations (\*\*) linear in terms of the unknown strengths σ?
- No! The left hand sides are, but the right hand side contains Q<sub>m</sub><sup>2</sup>, and Q<sub>m</sub> is linear in terms of the σ's
- Solve iteratively, given an initial guess as to inflow along the arms.

## Comparison to 3D: Horiz. Well

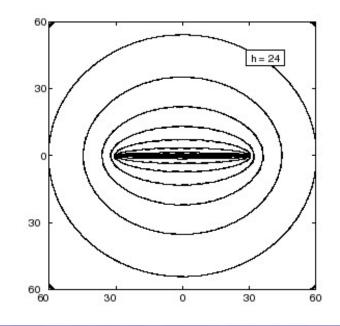
• center at (0,0); length = 60m

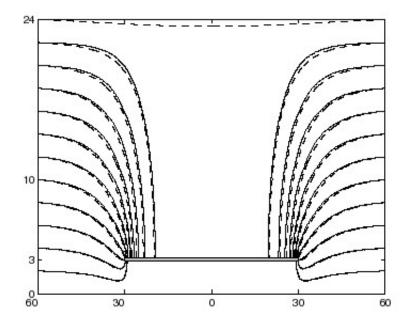
- elevation = 3m, radius = 0.15m
- total discharge = 12,000 m<sup>3</sup>/d
- IO line sinks, 6m each
- $\phi_0 = 24 \text{ m at } (60,0)$
- k = 150 m/d; unconfined
- "aquifer" = 12 layers; well in layer 8
- constant transmissivity in layer 1
- no head loss, etc.



Layer 1 Head (phreatic sfc) Dashed - ML Solid - 3D

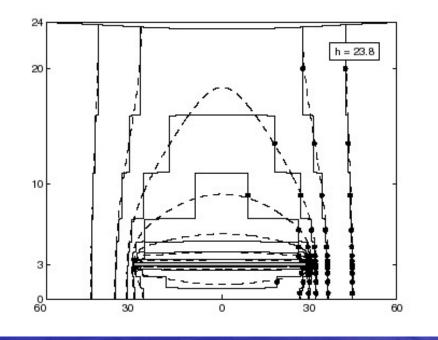
Layer 8 Head at well Dashed - ML Solid - 3D





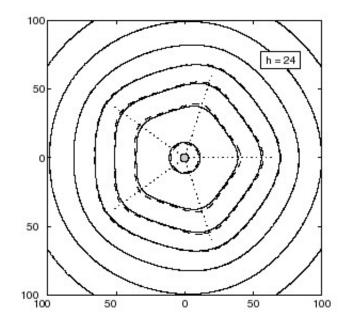
Streamlines Dashed - ML Solid - 3D

Cross section: heads Dashed - ML Solid - 3D



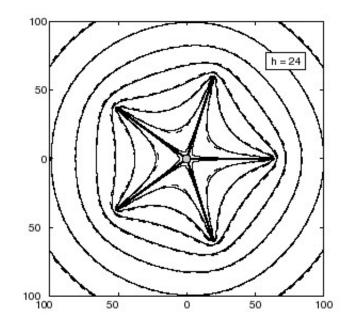
#### Comparison to 3D: RCW

• center at (0,0); 5 arms, length of each = 60m elevation = 3m, radius of each arm = 0.15m radius of caisson = 3m total discharge = 60,000 m<sup>3</sup>/d IO line sinks per arm, 6m each •  $\phi_0 = 24 \text{ m at } (100,0)$ • k = 150 m/d; unconfined • "aquifer" = 12 layers; well in layer 8 constant transmissivity in layer 1



Layer 1 Head (phreatic sfc) Dashed - ML Solid - 3D

Layer 8 Head at well Dashed - ML Solid - 3D



# Application

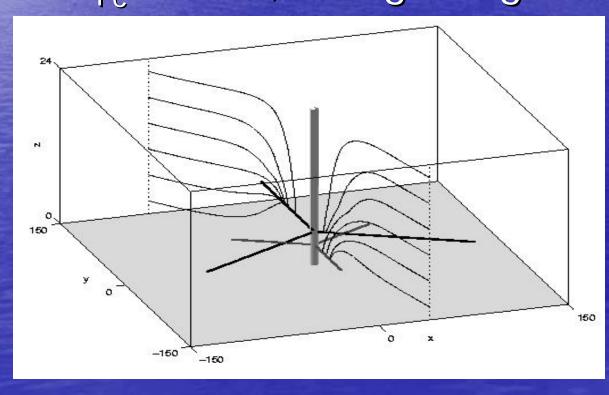
- (Slightly) new well geometry, aquifer properties
- Add laterals; investigate increase in well yield as a function of length of laterals and friction coefficient in Darcy-Weisbach equation

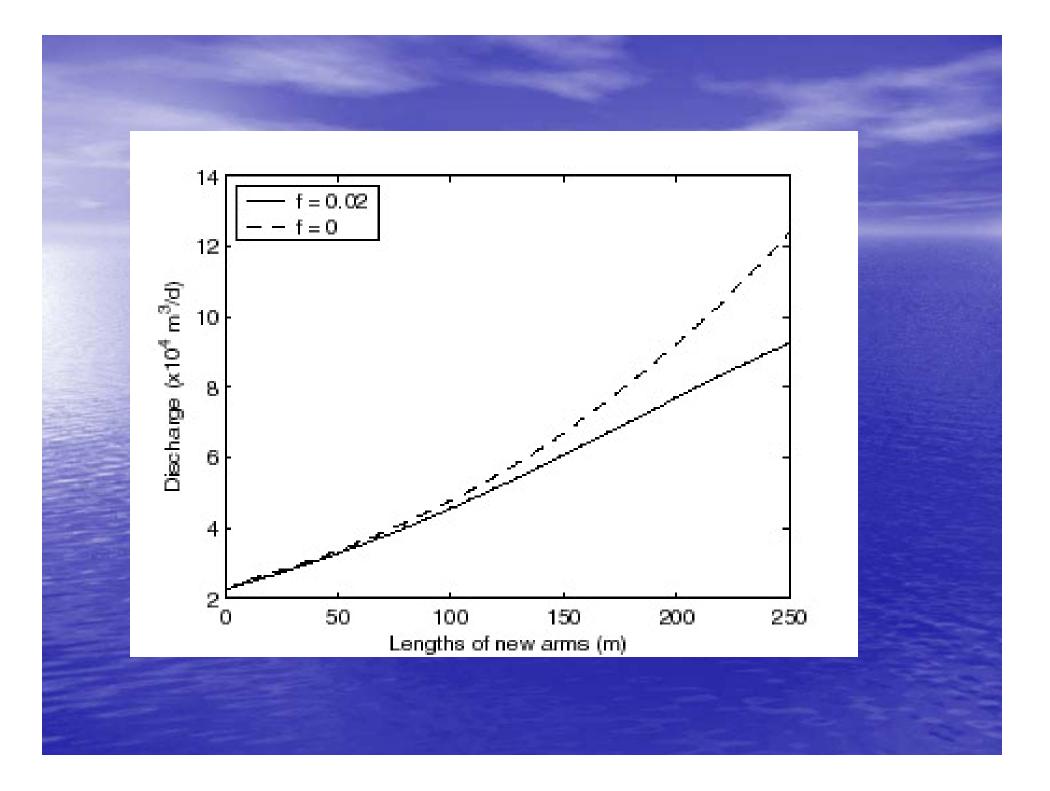
# Well / Aquifer

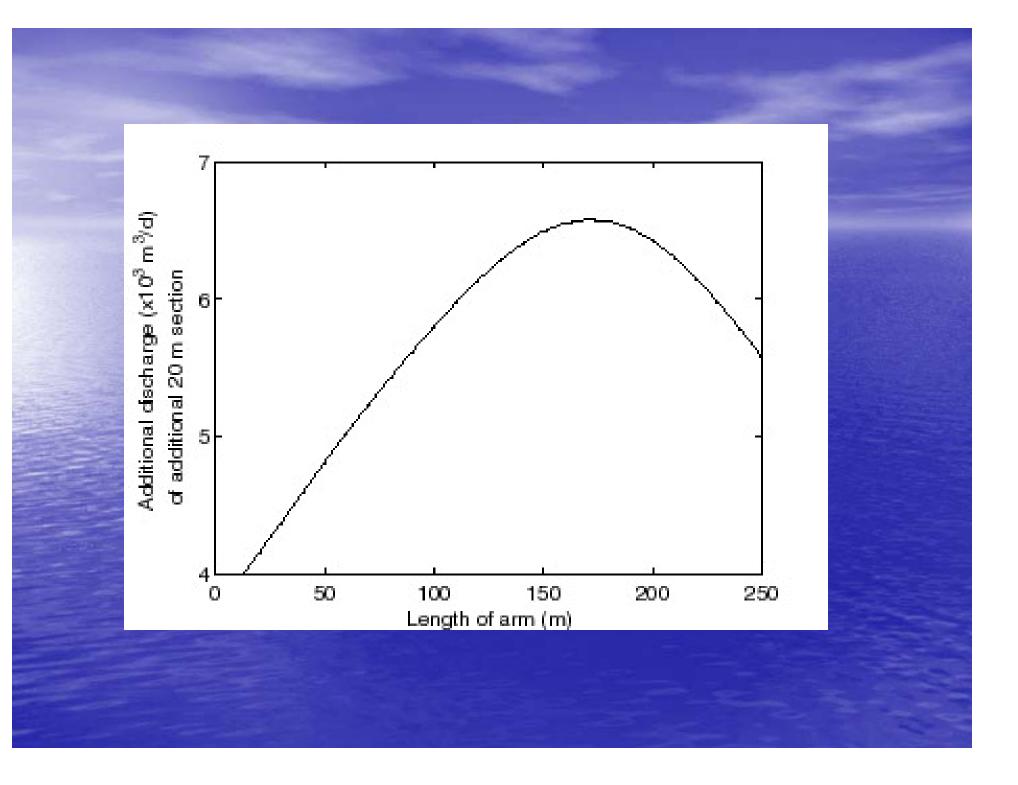
- center at (0,0); 3 arms, length of each = 60m
- elevation = 3m, radius of each arm = 0.15m
- radius of caisson = 3m; f = 0.02
- specified head in calsson  $\phi_c = 20m$
- $\phi_0 = 24 \text{ m at } (200,0)$
- k<sub>h</sub> = 200 m/d in bottom 10m; 100 m/d above
- $k_v = 60 \text{ m/d}$
- "aquifer" = 18 layers; well in layer 14
- RESULT: Q = 22,400 m<sup>3</sup>/d

#### Modifications to well

Add 3 arms in layer 9; skewed position
 Maintain \$\ointeq\_c\$ = 20m, change lengths







#### Summary

- Multilayer modeling of radial collector wells rules!
- Vertical stratification / anisotropy
- Boundary conditions along arm included
   (friction factor bac clarificant offect on wall
  - (friction factor has significant effect on well yield)
- Can be combined with regional 2D model